

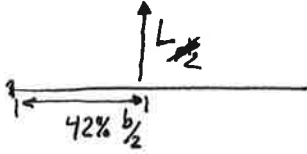
Comparison of Elliptical Loading and Cosine Approximation

Elliptical

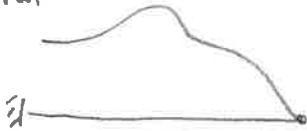


$$L' = \frac{L}{b} \frac{4}{\pi} \sqrt{1 - \left(\frac{2y}{b}\right)^2}$$

Equivalent Load



Polynomial



$$L' = A + By + Cy^2 + Dy^3$$

Fourier (as seen in AEM 313)



$L' = \text{cosine terms}$

Numerical / Experimental



$$V_0 = - \int_0^{b/2} L' dy = \text{Numerically Integrate } L'$$

$$M_0 = \int_0^{b/2} L' y dy$$

y	L'
0	0
b/2	0

Cosine



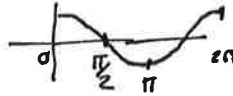
$$L' = A \cos\left(\frac{c}{b} y\right)$$

Boundary Conditions

$$L'(y=b/2) = 0 \quad (\text{Aerodynamics})$$

$$L' = A \cos\left(\frac{c}{b/2}\right) = 0$$

$$\cos\left(\frac{\pi}{2}\right) = 0 \Rightarrow$$



$$\frac{c}{b/2} = \frac{\pi}{2} \Rightarrow c = \frac{\pi}{b}$$

And the total force is $L/2$

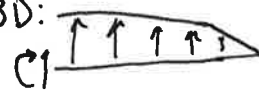
$$\frac{L}{2} = \int_0^{b/2} L' dy = \int_0^{b/2} A \cos\left(\frac{\pi}{b} y\right) dy$$

$$\frac{L}{2} = \frac{b}{\pi} A \Rightarrow A = \frac{L\pi}{2b}$$

Total:

$$L' = \frac{L\pi}{2b} \cos\left(\frac{\pi}{b} y\right)$$

FBD:



$$V_0 + \int_0^{b/2} L' dy = 0$$

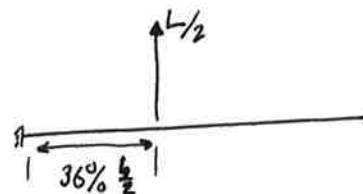
$$V_0 = -L$$

$$M_0 - \int_0^{b/2} L' y dy = 0$$

$$M_0 = b^2 \left(\frac{1}{2\pi} - \frac{1}{\pi^2} \right) \frac{L\pi}{2b}$$

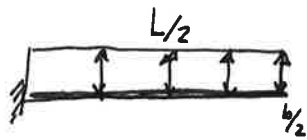
$$= \frac{L}{2} \frac{b}{2} 2\pi \left(\frac{1}{2\pi} - \frac{1}{\pi^2} \right) = 0.36 \frac{L}{2} \frac{b}{2}$$

Shear + Moment

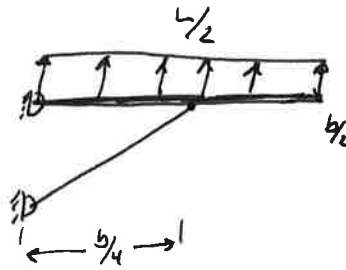


Not conservative. Rest moment too low

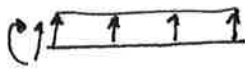
Ex: Compare the root bending moment of a cantilever and strut braced wing. Use a uniform loading ^{and maximum}



VS



Cantilever:

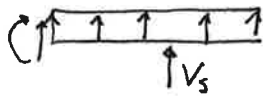


$$V_0 = -\frac{L}{2}$$

$$M_0 = -\frac{L}{2} \cdot \frac{b}{4} = -\frac{Lb}{8}$$



Strut:



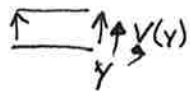
$$\sum F_z = 0 = V_0 + \frac{L}{2} + V_s$$

$$\sum M = 0 = M_0 + \frac{L}{2} \cdot \frac{b}{4} + V_s \cdot \frac{b}{4} \Rightarrow V_s = -\frac{L}{2}$$

subst' into $\sum F_z$

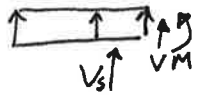
$$V_0 = \frac{L}{2} - \frac{L}{2} = 0 \quad \text{and} \quad M_0 = 0$$

Shear + Moment



$$V(y) + \frac{L}{2} \frac{2}{b} y = 0 \Rightarrow V_y = -\frac{L}{b} y$$

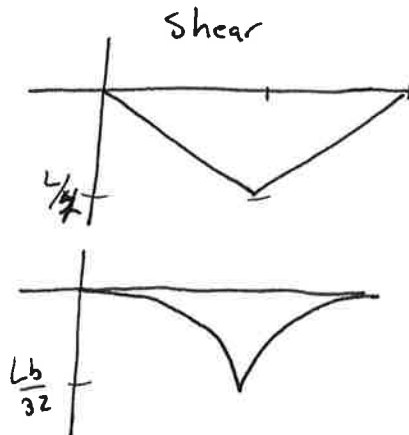
$$-M(y) - \int_0^y \left(\frac{L}{2} \frac{2}{b}\right) y dy = 0 \Rightarrow +M(y) = -\frac{L}{b} \frac{1}{2} y^2$$



$$V(y) + V_s + \frac{L}{2} \frac{2}{b} y = 0 \quad V(y) = -V_s - \frac{L}{b} y = \frac{L}{2} - \frac{L}{b} y$$

$$-M(y) - V_s \frac{b}{4} - \int_0^y \left(\frac{L}{2} \frac{2}{b}\right) y dy = 0$$

$$M(y) = -\frac{V_s b}{4} - \frac{L}{b} \frac{1}{2} y^2 = \frac{Lb}{8} - \frac{1}{2} \frac{L}{b} y^2$$



Pro:

Shear is half
Moment is 1/4

Con:

Wing has compressive load on inboard section from ~~the~~ drag of strut.

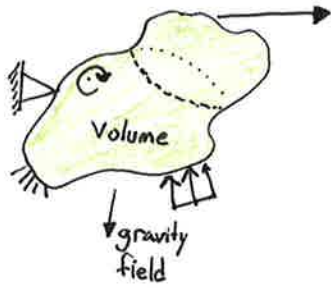
Aerospace Structural Design

3 critical requirements ... Given a set of loads

- Stress σ_{ij}
- Deformation ϵ_{ij} and δ
- Stability $\frac{d(\cdot)}{dt} \xrightarrow{\text{lim}} \infty$

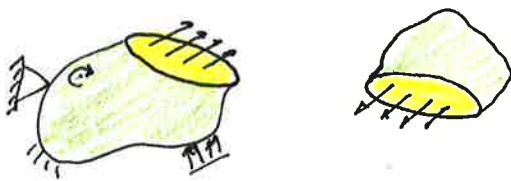
} Need to know stress and strain to evaluate failure criteria

Stress



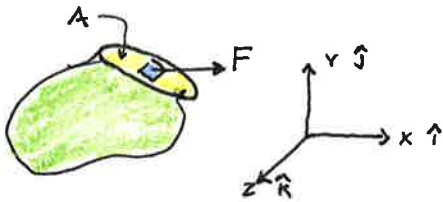
Subject to an arbitrary body/structure to loads, all physical objects will deform and create internal forces/loadings/stress fields.

If we cut the body with an arbitrary plate, there will be internal reactions

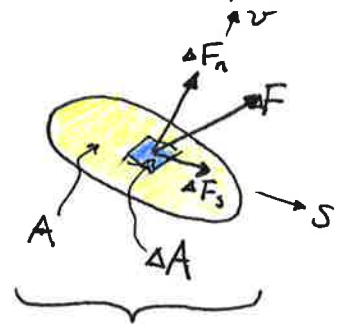


The removed piece will have equal but opposite reactions due to Newton's law.

At a small surface area on the cut, there is an internal reaction force F



which can be resolved into coordinate forces or in terms of normal and tangential forces on the cut, where ν is the normal direction



A surface traction vector is the ~~limit~~ force per unit area in the limit in a direction

$$T(\nu) = \lim_{\Delta A \rightarrow 0} \frac{\Delta F}{\Delta A} = \frac{dF}{dA}$$

Also write in terms of coordinate

$$T(\nu) = T_x \hat{i} + T_y \hat{j} + T_z \hat{k} = T$$

Given the plane cut's normal vector ν :

The component of A in the x direction is.

$$A_x = A \nu_x$$

and
$$A_y = A \nu_y$$

$$A_z = A \nu_z$$

Normal and Shear Stress

Normal $\sigma_n = \lim_{\Delta A \rightarrow 0} \frac{\Delta F_n}{\Delta A} = \frac{dF_n}{dA} = \underbrace{T \cdot \nu}_{\text{"pressure in a direction"}}$

Remember that the dot product takes the component of a vector in a particular direction. In this case T in the normal direction ν .

Shear

$$\sigma_s = \lim_{\Delta A \rightarrow 0} \frac{\Delta F_s}{\Delta A} = \frac{dF_s}{dA}$$

but $\Delta F = \Delta F_n \nu + \Delta F_s s$

vector scalar vector scalar vector

$$dF = dF_n \nu + dF_s s$$

$$\sigma_s = \frac{dF_s}{dA}$$

$$dF_s s = dF - dF_n \nu$$

But we can't divide by a vector s to get only dF_s . Rather, we will compute $\sigma_s s$

$$\sigma_s s = \frac{dF_s}{dA} s = \frac{1}{dA} (dF - dF_n \nu) s = \left(\underbrace{\frac{dF}{dA}}_T - \underbrace{\frac{dF_n}{dA} \nu}_{T \cdot \nu} \right) s$$

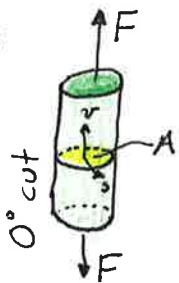
So the absolute value of σ_s is

$$|\sigma_s| = |T - (T \cdot \nu) \nu|$$

Total Traction vector minus the normal part

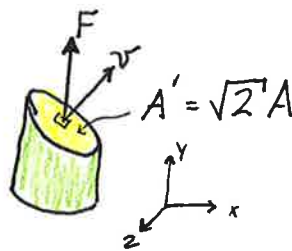
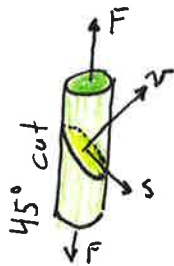
The normal and shear stresses depend on the surface orientation

Ex:



$$\sigma_n = \frac{F}{A}$$

$$\sigma_s = 0$$



$$T(\nu) = \lim_{\Delta A \rightarrow 0} \frac{\Delta F}{\Delta A}$$

$$= \frac{P}{\sqrt{2}} \frac{1}{A} \hat{j} = \frac{1}{\sqrt{2}} \frac{P}{A} \hat{j}$$

Compute $\nu = \frac{1}{\sqrt{2}} \hat{i} + \frac{1}{\sqrt{2}} \hat{j}$

• From above, $\sigma_n = T \cdot \nu = (0, \frac{1}{\sqrt{2}} \frac{P}{A}, 0) \cdot (\frac{1}{\sqrt{2}}, \frac{1}{\sqrt{2}}, 0)$

$$\sigma_n = \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}} \frac{P}{A} = \frac{1}{2} \frac{P}{A} = \sigma_n$$

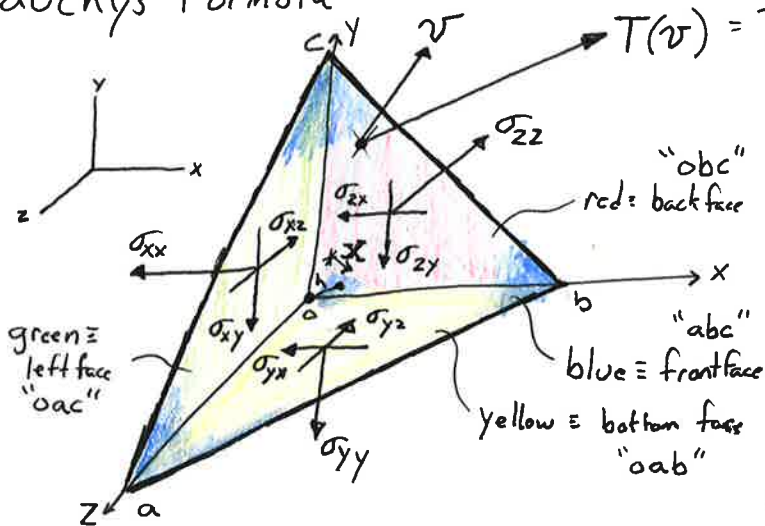
• Compute $\sigma_s = |T - (T \cdot \nu) \nu|$

$$\sigma_s = \left| \frac{1}{\sqrt{2}} \frac{P}{A} \hat{j} - \left(\frac{1}{2} \frac{P}{A} \right) \left(\frac{1}{\sqrt{2}} \right) (\hat{i} + \hat{j}) \right| = \left| \frac{1}{2\sqrt{2}} \frac{P}{A} \hat{i} + \frac{1}{2\sqrt{2}} \frac{P}{A} \hat{j} \right|$$

$$\sigma_s = \frac{1}{2} \frac{P}{A}$$

This is just stress transformation

Cauchy's Formula



$$T(v) = T_x \hat{i} + T_y \hat{j} + T_z \hat{k}$$

Volume of tetrahedra

$$V = \frac{1}{3} A \cdot h$$

Area of faces

$$A_{abc} = A$$

$$A_{oac} = A v_x$$

$$A_{oab} = A v_y$$

$$A_{obc} = A v_z$$

Summation of forces in x-direction

$$\sum F_x = 0 = \underbrace{T_x A}_{\text{front face}} - \underbrace{\sigma_{xx} A_{oac}}_{\text{left face}} - \underbrace{\sigma_{yx} A_{oab}}_{\text{bottom}} - \underbrace{\sigma_{zx} A_{obc}}_{\text{back}} + \underbrace{\rho V}_{\text{body force}} = 0$$

← static

$$= T_x A - \sigma_{xx} A v_x - \sigma_{yx} A v_y - \sigma_{zx} A v_z + \rho \frac{1}{3} A h = 0$$

The area "A" is common, so apparently the size does not matter. Thus, as $h \rightarrow 0$

$$T_x = \sigma_{xx} v_x + \sigma_{yx} v_y + \sigma_{zx} v_z$$

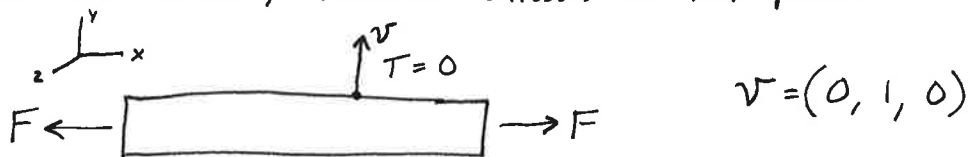
Likewise,

$$T_y = \sigma_{xy} v_x + \sigma_{yy} v_y + \sigma_{zy} v_z$$

$$T_z = \sigma_{xz} v_x + \sigma_{yz} v_y + \sigma_{zz} v_z$$

These Cauchy equations are ~~required~~ required to be true on any surface of a body (internal or natural external),

Ex: Given a body in uniaxial loading, and NO traction at a point on the surface, determine stresses at that point.



$$v = (0, 1, 0)$$

$$T_x = 0 = \sigma_{xx} v_x + \sigma_{yz} v_y + \sigma_{zx} v_z = \sigma_{xx} \cdot 0 + \underbrace{\sigma_{yz}} \cdot 1 + \sigma_{zx} \cdot 0$$

Thus, σ_{yz} must be zero

$$T_y = 0 = \sigma_{xy} v_x + \underbrace{\sigma_{yy}} v_y + \sigma_{zy} v_z$$

$$\sigma_{yy} = 0$$

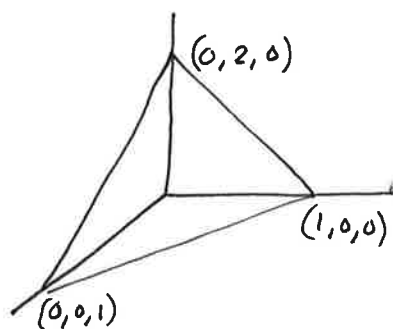
$$T_z = 0 = \sigma_{xz} v_x + \underbrace{\sigma_{yz}} v_y + \sigma_{zz} v_z$$

$$\sigma_{yz} = 0$$

Notice this is on the surface.

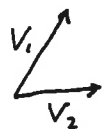
Ex: 2.1

$$\sigma = \begin{bmatrix} 1 & 0 & -4 \\ 0 & 3 & 0 \\ -4 & 0 & 5 \end{bmatrix} = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} \end{bmatrix} \quad \text{and}$$



⊙ Find v

The cross product is a good way to generate normals. pick 2 vectors on the surface.



$$v_1 = (0, 2, 0) - (0, 0, 1) = (0, 2, -1)$$

$$v_2 = (1, 0, 0) - (0, 0, 1) = (1, 0, -1)$$

$$v = \frac{v_2 \times v_1}{|v_2 \times v_1|} = \frac{\begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ 1 & 0 & -1 \\ 0 & 2 & -1 \end{vmatrix}}{\text{magnitude}} = \frac{2\hat{i} + \hat{j} + 2\hat{k}}{\sqrt{2^2 + 1^2 + 2^2}} = \boxed{\frac{2}{3}\hat{i} + \frac{1}{3}\hat{j} + \frac{2}{3}\hat{k}}$$

a) Find T components on plane described by v

$$T_x = \sigma_{xx} v_x + \sigma_{yx} v_y + \sigma_{zx} v_z = 1 \cdot \frac{2}{3} + 0 \cdot \frac{1}{3} + -4 \cdot \frac{2}{3} = \frac{2}{3} - \frac{8}{3} = -\frac{6}{3} = -2$$

$$T_y = \sigma_{xy} v_x + \sigma_{yy} v_y + \sigma_{zy} v_z = 0 \cdot \frac{2}{3} + 3 \cdot \frac{1}{3} + 0 \cdot \frac{2}{3} = 1$$

$$T_z = \sigma_{xz} v_x + \sigma_{yz} v_y + \sigma_{zz} v_z = -4 \cdot \frac{2}{3} + 0 \cdot \frac{1}{3} + 5 \cdot \frac{2}{3} = -\frac{8}{3} + \frac{10}{3} = \frac{2}{3}$$

$$\boxed{Tv = \left(-2, 1, \frac{2}{3}\right)}$$

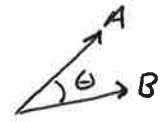
b) Find the magnitude of T

$$|T| = \sqrt{(-2)^2 + 1^2 + \left(\frac{2}{3}\right)^2} = \sqrt{4 + 1 + \frac{4}{9}} = \frac{7}{3}$$

c) Angle between T and v

Use dot product:

~~Use dot product:~~ $A \cdot B = |A||B| \cos \theta$



$$T(v) \cdot v = |T||v| \cos \theta$$

$$\left(-2, 1, \frac{2}{3}\right) \cdot \left(\frac{2}{3}, \frac{1}{3}, \frac{2}{3}\right) = \frac{7}{3} \cdot 1 \cdot \cos \theta$$

$$-2 \cdot \frac{2}{3} + \frac{1}{3} + \frac{4}{9} = -\frac{5}{9} = \frac{7}{3} \cos \theta \Rightarrow \cos \theta = -\frac{15}{63}$$

$$\theta = 76^\circ$$

d) $N = T \cdot v$
 $= -\frac{5}{9}$

e) $S = |T - (N)v| = \left| \left(-2, 1, \frac{2}{3}\right) - \left(-\frac{5}{9}\right)\left(\frac{2}{3}, \frac{1}{3}, \frac{2}{3}\right) \right|$
 $= \left| \left(-2 + \frac{10}{27}, 1 + \frac{5}{27}, \frac{2}{3} + \frac{2}{3}\right) \right|$

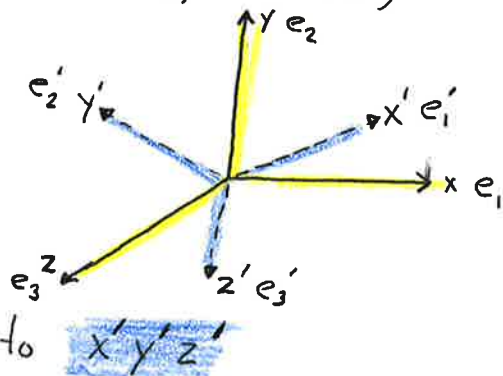
This gets messy

Stress Transformation

Stress is a 2nd order tensor, not a vector, you cannot transform a stress by transforming each component. (i.e. $\sigma_{xx} \neq \sigma_{xx} \cos \theta + \sigma_{yy} \sin \theta$ etc)

Given a stress state.

$$\sigma = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} \end{bmatrix}$$



Convert/Transform from coordinates **xyz** to **x'y'z'**

Define $A_{ij} \equiv e'_i \cdot e_j = \cos(\text{angle between } e'_i \text{ and } e_j)$

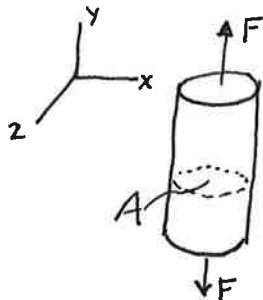
e are unit vectors

The transformation is

$$\sigma' = A \sigma A^T$$

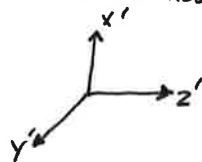
$$\begin{bmatrix} \sigma'_{x'x'} & \sigma'_{x'y'} & \sigma'_{x'z'} \\ \sigma'_{x'y'} & \sigma'_{y'y'} & \sigma'_{y'z'} \\ \sigma'_{x'z'} & \sigma'_{y'z'} & \sigma'_{z'z'} \end{bmatrix} = \begin{bmatrix} A_{11} & A_{12} & A_{13} \\ A_{21} & A_{22} & A_{23} \\ A_{31} & A_{32} & A_{33} \end{bmatrix} \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} \end{bmatrix} \begin{bmatrix} A_{11} & A_{21} & A_{31} \\ A_{12} & A_{22} & A_{32} \\ A_{13} & A_{23} & A_{33} \end{bmatrix}$$

Ex: Uniaxial Loading



$$\sigma = \begin{bmatrix} 0 & 0 & 0 \\ 0 & F/A & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

Switch to new coordinate system



$$\begin{aligned} A_{11} &= e'_1 \cdot e_x = 0 & A_{23} &= e'_2 \cdot e_z = 1 \\ A_{12} &= e'_1 \cdot e_y = 1 & A_{31} &= e'_3 \cdot e_x = 1 \\ A_{13} &= e'_1 \cdot e_z = 0 & A_{32} &= e'_3 \cdot e_y = 0 \\ A_{21} &= e'_2 \cdot e_x = 0 & A_{33} &= e'_3 \cdot e_z = 0 \\ A_{22} &= e'_2 \cdot e_y = 0 \end{aligned}$$

Transformation

$$\sigma' = A \sigma A^T =$$

$$A = \begin{bmatrix} 0 & 1 & 0 \\ 0 & 0 & 1 \\ 1 & 0 & 0 \end{bmatrix}$$

$$= \begin{bmatrix} 0 & 1 & 0 \\ 0 & 0 & 1 \\ 1 & 0 & 0 \end{bmatrix} \begin{bmatrix} 0 & 0 & 0 \\ 0 & F/A & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} 0 & 0 & 1 \\ 1 & 0 & 0 \\ 0 & 1 & 0 \end{bmatrix}$$

$$= \begin{bmatrix} 0 & F/A & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} 0 & 0 & 1 \\ 1 & 0 & 0 \\ 0 & 1 & 0 \end{bmatrix}$$

$$= \begin{bmatrix} F/A & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} = \sigma'$$

Neat!