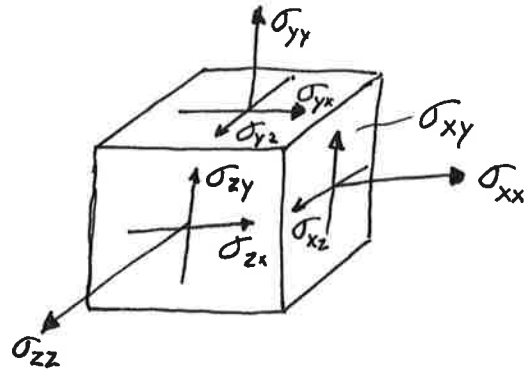


Three Dimensional Stress Analysis (principal and max shear stress)

Given a 3D stress state

$$\sigma = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix}$$

Symmetric



The principal stresses are the eigenvalues of σ

The max shear is $\frac{1}{2}$ the difference between principal stresses

The principal plane directions are the eigenvectors of σ

Ex:

$$\sigma = \begin{bmatrix} 10 & -4 & 0 \\ -4 & 6 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

Find principal stresses and max shear

① Eigenvalues of σ

$$|\sigma - I\lambda| = 0 \Rightarrow \begin{vmatrix} -\lambda + 10 & -4 \\ -4 & -\lambda + 6 \end{vmatrix} = \lambda^2 - \lambda(10+6) + 60 - 16$$

$$\sigma_1 = 12.47$$

$$\sigma_2 = \cancel{12.47} 3.53$$

$$\lambda = \frac{-b \pm \sqrt{b^2 - 4ac}}{2a} = \frac{16 \pm \sqrt{256 - 4 \cdot 44}}{2}$$

$$= 8 \pm 4.47$$

② Max Shear

$$\sigma_s = \frac{1}{2} (12.47 - \cancel{12.47}) = 4.47$$

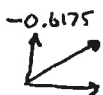
③ Direction (eigenvectors)

$$\sigma v = \lambda v$$

$$\begin{bmatrix} 10 & -4 \\ -4 & 6 \end{bmatrix} \begin{pmatrix} v_1 \\ v_2 \end{pmatrix} = 12.47 \begin{pmatrix} v_1 \\ v_2 \end{pmatrix} \Rightarrow 10v_1 - 4v_2 - 12.47v_1 = 0$$

v can be scaled, so let's pick $v_1 = 1$

$$v_{\sigma_1} = \begin{pmatrix} 1 \\ -0.6175 \end{pmatrix}$$



$$-2.47 = 4v_2 \Rightarrow v_2 = -0.6175$$

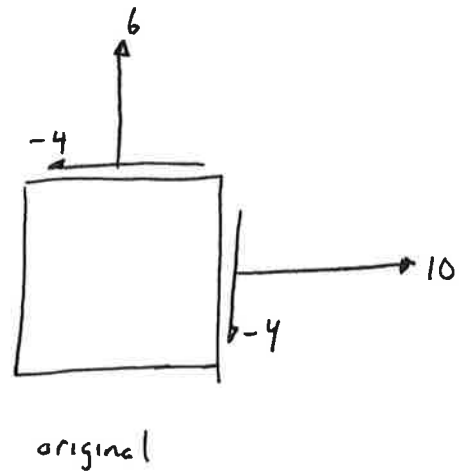
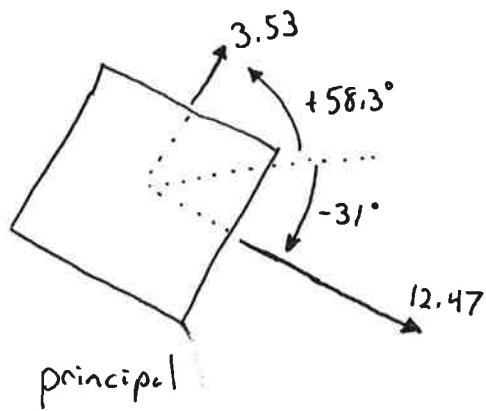
$$\phi = \arctan\left(\frac{-0.6175}{1}\right) = -31.7^\circ$$

And $10v_1 - 4v_2 = 3.53v_1 \Rightarrow 4v_2 = 6.47 \Rightarrow v_2 = 1.6175$

$$v_{\sigma_2} = \begin{pmatrix} 1 \\ 1.6175 \end{pmatrix}$$

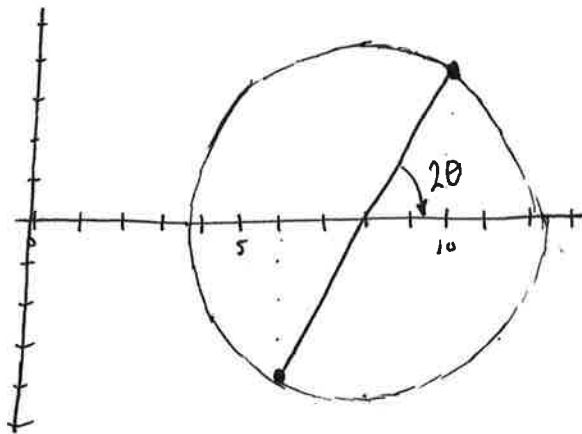
$$\phi = \arctan(1.6175) = 58.3^\circ$$

④ Draw prin' stress



⑤ Mohr's Circle

Notice that MC is only applicable because σ is planar stress
MC can not transform arbitrary 3D stresses.



① Mark $(10, 4)$
 $(6, -4)$

② midpoint is $(8, 0)$

③ Radius is $\sqrt{(10-8)^2 + 4^2}$
 $= 4.47$

④ principal stresses

$$\begin{aligned} 8+r &= 12.47 \\ 8-r &= 3.53 \end{aligned}$$

⑤ Max Shear

$$r = 4.47$$

⑥ principal angle

$$-2\theta = \text{atan}\left(\frac{4}{10-8}\right) = \text{atan} 2$$

$$-\theta = 31.7$$

$$\boxed{\theta = -31.7^\circ}$$

Same solution as
more general 3D
eigenvalue method

Alternative Method to calculate eigenvalues of 3×3 real sym matrix
By hand!

$$P_1 = \sigma_{xy}^2 + \sigma_{xz}^2 + \sigma_{yz}^2$$

$$q = \frac{1}{3}(\sigma_{xx} + \sigma_{yy} + \sigma_{zz})$$

$$P_2 = (\sigma_{xx} - q)^2 + (\sigma_{yy} - q)^2 + (\sigma_{zz} - q)^2 + 2P_1$$

$$p = \sqrt{P_2/6}$$

$$[B] = [A - q[I]] = \begin{bmatrix} \sigma_{xx} - q & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} - q & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} - q \end{bmatrix}$$

$$r = \frac{\det(B)}{2p^3}$$

$$\phi = \frac{\arccos(r)}{3}$$

$$\sigma_{p_1} = q + 2p \cos \phi$$

$$\sigma_{p_2} = q + 2p \cos \left(\phi + \frac{2}{3}\pi \right)$$

$$\sigma_{p_3} = q + 2p \cos \left(\phi + \frac{4}{3}\pi \right)$$

Calculator

eigval([σ])

- be careful, most calculators give eigenvectors that might not correspond to the eigenvalue order.

Ex: $\sigma = \begin{bmatrix} 1 & 2 & 3 \\ 2 & 4 & 6 \\ 3 & 6 & 1 \end{bmatrix} \times 10^4 \text{ psi}$ If failure occurs at 80 ksi, is the stress state safe?



$$p_1 = 2^2 + 3^2 + 6^2 = 4 + 9 + 36 = 49$$

$$q = \frac{1}{3}(1+4+1) = \frac{6}{3} = 2$$

$$p_2 = (1-2)^2 + (4-2)^2 + (1-2)^2 + 2 \cdot 49 = 1 + 4 + 1 + 98 = 104$$

$$p = \sqrt{104/6} = 4.163$$

$$B = \begin{bmatrix} -1 & 2 & 3 \\ 2 & 2 & 6 \\ 3 & 6 & -1 \end{bmatrix} \quad \text{only subtract } q \text{ on the diagonals!}$$

$$\det B = -1(-2-36) - 2(-2-18) + 3(12-6) = 38 + 40 + 18 = 96$$

$$r = 96/2 \cdot \frac{1}{4.163^3} = 0.665 \quad \text{be sure use divide by } p^3!$$

$$\phi = \frac{\arccos(0.665)}{3} = 0.281 \quad \text{this is in radians}$$

$$\sigma_{p_1} = 2 + 2 \cdot 4.163 \cdot \cos(0.281) = 10$$

$$\sigma_{p_2} = 2 + 2 \cdot 4.163 \cdot \cos\left(0.281 + \frac{2}{3}\pi\right) = -4$$

$$\sigma_{p_3} = 2 + 2 \cdot 4.163 \cdot \cos\left(0.281 + \frac{4}{3}\pi\right) = 0$$

principal stresses are 10, 4, 0

$$10 > 8$$

Failure of material
Not Safe

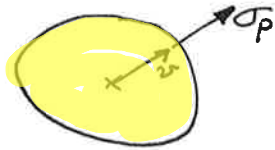
Use a calculator or computer when available.

You can use eigenvectors to find principal directions ... by hand is possible.

Why ^{are} the eigenvalues the principal stresses?

① Definition of principal stress: Stress state where the shear is zero

σ_p on a principal plane of normal ν



② Traction Vector

What is the traction vector associated with σ_p along ν ?

$$T(\nu) = \sigma_p \nu$$

Notice, no shear so $T(\nu)$ is aligned with ν .

$$T(\nu) = T_x \nu_x \hat{i} + T_y \nu_y \hat{j} + T_z \nu_z \hat{k} \\ = \sigma_p \nu_x \hat{i} + \sigma_p \nu_y \hat{j} + \sigma_p \nu_z \hat{k}$$

③ Cauchy equations

x: $\underbrace{T_x(\nu)}_{\sigma_p \nu_x} = \sigma_{xx} \nu_x + \sigma_{xy} \nu_y + \sigma_{xz} \nu_z$ in \uparrow direction

Thus

$$(\sigma_{xx} - \sigma_p) \nu_x + \sigma_{xy} \nu_y + \sigma_{xz} \nu_z = 0$$

y: $\underbrace{T_y(\nu)}_{\sigma_p \nu_y} = \sigma_{xy} \nu_x + \sigma_{yy} \nu_y + \sigma_{yz} \nu_z$

Thus

$$\sigma_{xy} \nu_x + (\sigma_{yy} - \sigma_p) \nu_y + \sigma_{yz} \nu_z = 0$$

z: $\underbrace{T_z(\nu)}_{\sigma_p \nu_z} = \sigma_{xz} \nu_x + \sigma_{yz} \nu_y + \sigma_{zz} \nu_z$

Thus

$$\sigma_{xz} \nu_x + \sigma_{yz} \nu_y + (\sigma_{zz} - \sigma_p) \nu_z = 0$$

Put these 3 equations together and pull out v_x, v_y, v_z

$$\begin{bmatrix} \sigma_{xx} - \sigma_p & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} - \sigma_p & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} - \sigma_p \end{bmatrix} \begin{pmatrix} v_1 \\ v_2 \\ v_3 \end{pmatrix} = 0$$

either this is zero

or this is zero

④ Recall the definition of an eigenvalue

- Given matrix "A", solve $|A - \lambda I| = 0$ where λ is an eigenvalue and v is an eigenvector
- ~~$A v = \lambda v$~~ $A v = \lambda v \Rightarrow A - \lambda I = 0$

⑤ Our green matrix above maps exactly to an eigenvalue problem where

$$A = \sigma = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} \end{bmatrix}$$

such that $A - \lambda I = [\sigma] - [\sigma_p] I$

$$= \begin{bmatrix} \sigma_{xx} - \sigma_p & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} - \sigma_p & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} - \sigma_p \end{bmatrix}$$

⑥ Eigenvalues of $[\sigma]$ are σ_p

Eigenvectors of $[\sigma]$ are principle directions of σ_p

$$\text{Also } |[\sigma] - \sigma_p [I]| = -\sigma_p^3 + I_1 \sigma_p^2 - I_2 \sigma_p + I_3 = 0$$

Which is where I_1, I_2, I_3 come from as calculated in textbook

$$I_1 = \sigma_{xx} + \sigma_{yy} + \sigma_{zz}$$

$$I_2 = \begin{vmatrix} \sigma_{xx} & \sigma_{xy} \\ \sigma_{xy} & \sigma_{yy} \end{vmatrix} + \begin{vmatrix} \sigma_{yy} & \sigma_{yz} \\ \sigma_{yz} & \sigma_{zz} \end{vmatrix} + \begin{vmatrix} \sigma_{zz} & \sigma_{xz} \\ \sigma_{xz} & \sigma_{xx} \end{vmatrix}$$

$$I_3 = \det(\sigma) = \begin{vmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} \end{vmatrix}$$

Ex: Shear Stress (max) of Principal Directions $\sigma = \begin{bmatrix} 1 & 2 & 3 \\ 2 & 4 & 6 \\ 3 & 6 & 1 \end{bmatrix}$ where $\sigma_p = (10, -4, 0)$

• $\sigma_p = 10$

$$([\sigma] - \sigma_p[I])v = 0$$

Need to find v such that this is true.
Infinite # choices actually!

$$\begin{bmatrix} 1-10 & 2 & 3 \\ 2 & 4-10 & 6 \\ 3 & 6 & 1-10 \end{bmatrix} \begin{pmatrix} v_1 \\ v_2 \\ v_3 \end{pmatrix} = \begin{cases} -9v_1 + 2v_2 + 3v_3 = 0 \\ 2v_1 - 6v_2 + 6v_3 = 0 \\ 3v_1 + 6v_2 - 9v_3 = 0 \end{cases}$$

Pick $v_{1x} = 1$ and solve for v_{1y} v_{1z}
This naturally has $\det = 0$ (infinite # solutions)

$$\begin{cases} 2v_2 + 3v_3 = 9 \\ -6v_2 + 6v_3 = -2 \end{cases} \Rightarrow \begin{bmatrix} 2 & 3 \\ -6 & 6 \end{bmatrix} \begin{pmatrix} v_{2y} \\ v_{2z} \end{pmatrix} = \begin{pmatrix} 9 \\ -2 \end{pmatrix}$$

This can be solved.

Inverse of 2×2 matrix $[M]$

- flip diagonal terms ($M'_{11} = M_{22}$)
- - off diagonals ($M'_{12} = -M_{12}$)
- Divide by $\det(M)$

$$\begin{pmatrix} v_{2y} \\ v_{2z} \end{pmatrix} = \begin{bmatrix} 2 & 3 \\ -6 & 6 \end{bmatrix}^{-1} \begin{pmatrix} 9 \\ -2 \end{pmatrix} = \frac{1}{12 + 18} \begin{bmatrix} 6 & -3 \\ 6 & 2 \end{bmatrix} \begin{pmatrix} 9 \\ -2 \end{pmatrix} = \frac{1}{30} \begin{pmatrix} 60 \\ 50 \end{pmatrix} = \begin{pmatrix} 2 \\ 5/3 \end{pmatrix}$$

$$v_1 = \begin{pmatrix} 1 \\ 2 \\ 5/3 \end{pmatrix} \text{ Normalize by } 1^2 + 2^2 + (5/3)^2 = \sqrt{7.777} = 2.788$$

$$v_1 = \begin{pmatrix} 0.358 \\ 0.717 \\ 0.597 \end{pmatrix} \text{ for } \sigma_p = 10$$

• $\sigma_p = -4$

$$v_2 = \begin{pmatrix} -0.894 \\ 0.447 \\ 0 \end{pmatrix}$$

• $\sigma_p = -4$

$$v_3 = \begin{pmatrix} -0.267 \\ -0.535 \\ 0.802 \end{pmatrix}$$

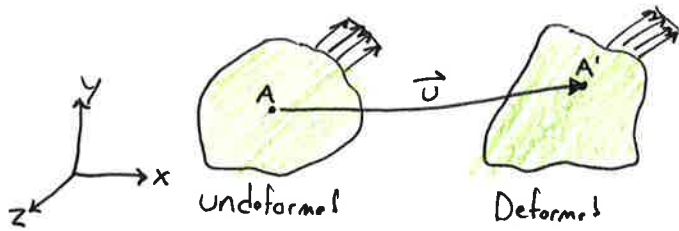
• Shear Stress

$$\sigma_{s1} = \left| \frac{10 - 4}{2} \right| = 3$$

$$\sigma_{s2} = \left| \frac{4 - 0}{2} \right| = 2$$

$$\sigma_{s3} = \left| \frac{10 - 0}{2} \right| = 5$$

Deformation Analysis



$$\vec{U} = U\hat{i} + V\hat{j} + W\hat{k}$$

\uparrow Vector \nwarrow scalar

Given the body is a continuum, $\vec{U} = \vec{U}(x, y, z)$

We can take spatial derivatives

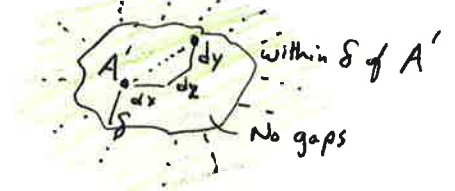
$$\begin{matrix} \frac{dU}{dx} & \frac{dU}{dy} & \frac{dU}{dz} \\ \frac{dV}{dx} & \frac{dV}{dy} & \frac{dV}{dz} \\ \frac{dW}{dx} & \frac{dW}{dy} & \frac{dW}{dz} \end{matrix}$$

Write the deformation of A to A'

$$A' = A + \vec{U}$$

If the body is a continuum, then in the nearby neighborhood of A

$$x \rightarrow x + U + \frac{dU}{dx} dx + \frac{dU}{dy} dy + \frac{dU}{dz} dz$$

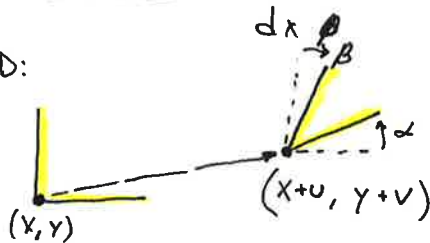


Engineering Strain

$$\epsilon_{xx} \approx \frac{\left(\frac{dU}{dx} dx + U + x\right) - (U + x)}{dx} = \frac{dU}{dx}$$

$$\epsilon_{xx} \equiv \frac{dU}{dx}$$

In 2D:



$$\epsilon_{xy} = \alpha + \beta = \frac{dV}{dx} + \frac{dU}{dy}$$

Visually

