

# Shear in advanced beams - part 2

The shear flow derivation has  $A_s^*$ ,  $Q_y^*$ ,  $Q_z^*$  which are integrated terms as a function of the cross section geometry, materials, and location.

$$A_s^* = \int_0^{s_1} \frac{E_i}{E_1} t ds \quad Q_y^* = \int_0^{s_1} \frac{E_i}{E_1} z t ds \quad Q_z^* = \int_0^{s_1} \frac{E_i}{E_1} y t ds$$

These can be a challenge to calculate. But for many actual as-built structures, we can simplify the integrals by assuming a summation of parts.



$$A_s^* = \int_0^{s_1} \frac{E_i}{E_1} t ds = \sum_{s_i=1}^{s_i} \int \frac{E_i}{E_1} t ds$$

composed of parts

$$= \sum \frac{E_i}{E_1} \int_{s_i=1}^{s_i} t ds = \sum \frac{E_i}{E_1} A_i$$

pull out constants since each piece has constant  $E_i$



$$Q_y^* = \int_0^{s_1} \frac{E_i}{E_1} z t ds = \sum_{s_i=1}^{s_i} \int \frac{E_i}{E_1} z t ds = \sum \frac{E_i}{E_1} \int z t ds = \sum \frac{E_i}{E_1} \int z dA$$

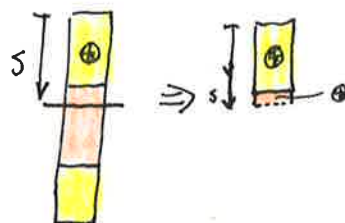
do you recognize  $\int z dA$ ?  $\bar{z} = \frac{1}{A} \int z dA$

$$Q_y^* = \sum \frac{E_i}{E_1} A_i \bar{z}_i$$

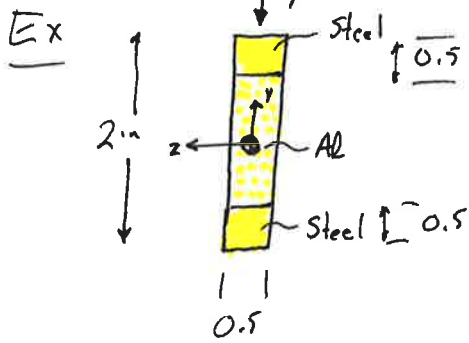
Likewise

$$Q_z^* = \sum \frac{E_i}{E_1} A_i \bar{y}_i$$

But remember that there may be a partial part at distance  $s$ .

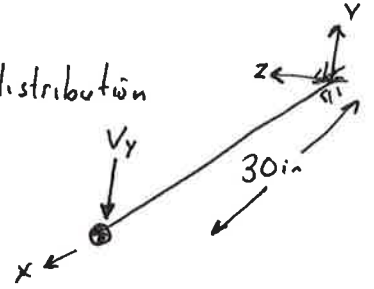


Still complicated, but doable



plot the shear stress distribution

$$E_1 = 10 \times 10^6 \text{ psi}$$



Part	$E/E_1$	$A_i$	$E_i/E_1 A_i$	$I_{z,z_0}$	$\bar{z}$	$\frac{E_i}{E_1} \bar{z} A$	$\frac{E_i}{E_1} (I_{z,z_0} + y^2 A)$
1	3	0.25	0.75	0.005208	0.75	0.5625	0.4375
2	1	0.5	0.5	0.041667	0	0	0.041667
3	3	0.25	0.75	0.005208	-0.75	-0.5625	0.4375
		$A^* = 2.0$				$\bar{z} = 0$	$I_{zz}^* = 0.91665$

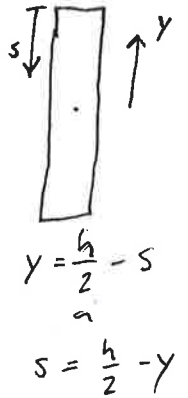
At root:  
 $M_y = -V_y \cdot 30$

$$q(s_i) = q(0) - p^T \frac{A_i^*}{A^*} + \left( \frac{(-M_y - V_y - M_y^T) I_{yy}^* + (\dots)}{I_{yy}^* I_{zz}^* - I_{yz}^{*2}} \right) Q_z^* - (\dots) Q_y^* + E_1 \int \frac{A_i^*}{A^*} ds$$

$$= q(0) - \frac{V_y Q_z^*}{I_{zz}^*}$$

$$Q_z^* = \sum \frac{E_i}{E_1} A_i \bar{z}_i = \begin{cases} 3 \cdot t \cdot s \cdot \left(\frac{h}{2} - \frac{s}{2}\right) & \text{in upper (1)} \\ 3 \cdot t \cdot \frac{h}{2} \left(\frac{h}{2} - \frac{h}{4}\right) + 1 \cdot t \cdot \left(s - \frac{h}{2}\right) (\dots) & \text{in (2)} \\ \dots & \text{in (3)} \end{cases}$$

What a mess!  
 There is an easier way



$$= \int \frac{E_i}{E_1} \bar{z} ds = \int_0^s \frac{E_i}{E_1} \left(\frac{h}{2} - s\right) ds$$

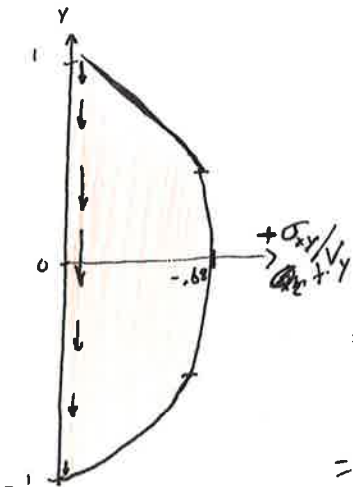
$$= \int_0^{0.5} 3 \left(\frac{h}{2} - s\right) ds + \int_{0.5}^{1.5} \left(\frac{h}{2} - s\right) ds + \int_{1.5}^s 3 \left(\frac{h}{2} - s\right) ds$$

$$= 1.5 \int_0^s (1-s) ds = 1.5 \left( s - \frac{s^2}{2} \right) \Big|_0^s = \left( s - \frac{s^2}{2} \right) 1.5 \quad \text{when } s < 0.5$$

$$= 1.5 \left( 0.5 - \frac{0.25}{2} \right) + 0.5 \int_{0.5}^s (1-s) ds = 0.5625 - 0.25s^2 + 0.5s - 0.1875 \quad \text{when } 0.5 < s < 1.5$$

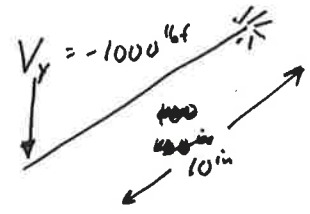
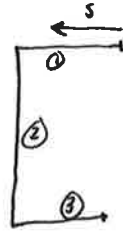
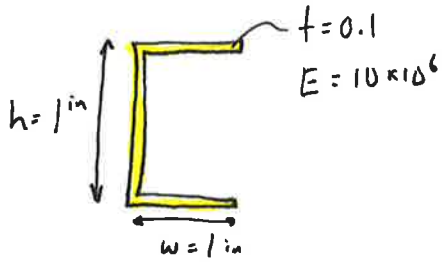
$$= 0.5625 + 0 + 1.5 \int_{1.5}^s (1-s) ds = -0.75s^2 + 1.5s - 0.5625 \quad \text{when } s > 1.5$$

$$= 0.5625 - 0.75s^2 + 1.5s - 0.5625$$



The integral is easier if you want the distribution, Summation is better for specific points

Ex: C channel

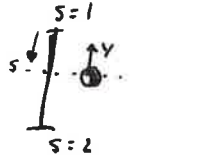


$$① Q_{z_1}^* = \int \frac{E}{E_1} y t ds = \int \frac{E}{E_1} y t ds = \int_0^{s_1} \underbrace{0.5}_{y} \cdot \underbrace{0.1}_{t} ds = 0.05 s$$

$$② Q_{z_2}^* \text{ in } | = \int y t ds = \int_1^s y t ds = \int_1^s (1.5-s) 0.1 ds = 0.05 + \left( 0.15s - \frac{0.1s^2}{2} \right) \Big|_1^s$$

$$= 0.05 + (0.15s - 0.05s^2 - 0.15 + 0.05)$$

$$= -0.05s^2 + 0.15s - 0.05$$



$$③ Q_{z_3}^* \text{ in } \text{---} = \int y t ds = \frac{E_1}{E} A_1 y_1 + \frac{E_2}{E} A_2 y_2 + \int_2^s y t ds = 0.05 + 0 + 0.1 \int_2^s -0.5 ds$$

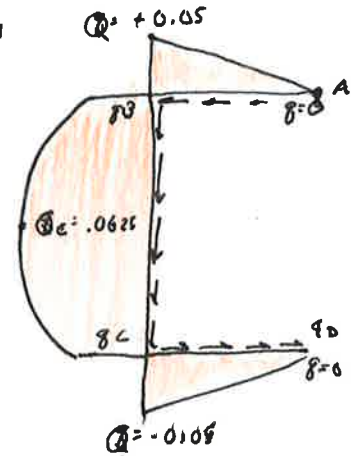
$$= 0.05 + -0.05(s-2) = 0.15 - 0.05s$$

$$I_{zz} = \underbrace{\frac{1}{12} 0.1 \cdot 1^3}_{I_{z_1 z_1}} + \underbrace{2 \cdot \frac{1}{12} 1 \cdot 0.1^3}_{2 \cdot I_{z_2 z_2}} + \underbrace{(0.1)(1)(2)}_A \cdot \underbrace{(0.5)^2}_{y^2} = 0.054433 \text{ in}^4$$

$$q(s) = q(v) - \frac{V_y Q_z^*}{I_{zz}} \Rightarrow \sigma_{xy} = \frac{V_y Q_z}{I_{zz} t}$$

$$\sigma_A = 0, \quad \sigma_B = \frac{1000 \cdot 0.05}{0.0585} = 854 \frac{\text{lb}}{\text{in}}$$

$$\sigma_C = \sigma_D, \quad \sigma_0 = 0, \quad \sigma_6 = 1068 \frac{\text{lb}}{\text{in}}$$



$$\sigma_{xy} = -\frac{V_y Q_z}{I_{zz} t} = \frac{1068}{0.1} = 10 \text{ ksi}$$

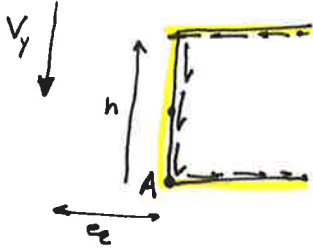
Compare to  $\sigma_{xx} = -\frac{M y}{I} = \pm 85 \text{ ksi}$

# Shear Center

The point on the cross section's plane where a shear load creates no twisting  
NOT ALWAYS THE CENTROID unless 2 symmetry planes

Ex:

The shear flow creates an applied torque/moment.



What is the moment about A due to shear flow?

$$M_A = \int g h ds = \int_0^1 \frac{V_y Q_z h}{I_{zz}} ds = \frac{V_y h}{I_{zz}} \int_0^1 0.05 s ds$$

$$= \frac{V_y}{I_{zz}} 0.05 \int_0^1 s ds = \frac{V_y}{I_{zz}} 0.05 \frac{1}{2} = \frac{V_y 0.05}{2} = 0.054433 V_y$$

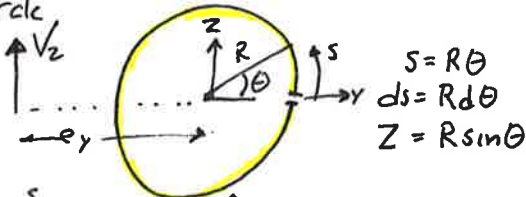
$$= 0.4593 V_y$$

What is the moment due to  $V_y$  in negative  $y$  direction at  $e_2$  from A?

$$M_A = V_y e_2$$

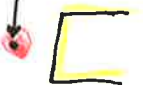
Where are these the same?  $V_y e_2 = 0.4593 V_y \Rightarrow e_2 = 0.4593$

Ex: Cut circle



NOT ON CROSS SECTION!!

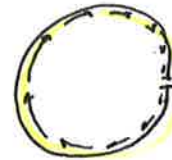
No twist



$$I_{yy} = \pi R^3 +$$

$$Q_y = \int_0^s z t ds = \int_0^\theta \underbrace{R \sin \theta}_z + \underbrace{R d\theta}_t ds = R^2 \int_0^\theta \sin \theta d\theta = R^2 (1 - \cos \theta)$$

$$g = \frac{-V_z R^2 (1 - \cos \theta)}{\pi R^3 +} = \frac{-V_z Q_y}{I_{yy}} = \frac{-V_z (1 - \cos \theta)}{\pi R}$$



What is the moment about center from shear flow?

$$M_0 = \int_0^{2\pi} g R ds = \int_0^{2\pi} \frac{-V_z}{\pi R} (1 - \cos \theta) R^2 d\theta = \frac{-V_z R}{\pi} (\theta - \sin \theta) \Big|_0^{2\pi} = \frac{-V_z R}{\pi} 2\pi = -V_z 2R$$

What is the moment of  $V_z$  at  $e_y$  from origin?

$$M_0 = -V_z e_y$$

Where are these the same?  $-V_z e_y = -V_z 2R \Rightarrow e_y = 2R$

Not dependant on  $V_z$ !

